



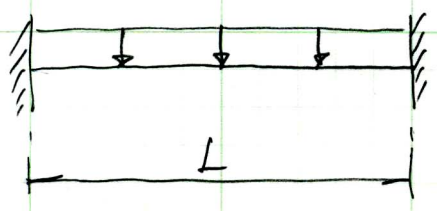
Chapter 4. Equilibrium Method

Introduction

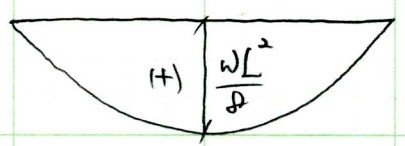
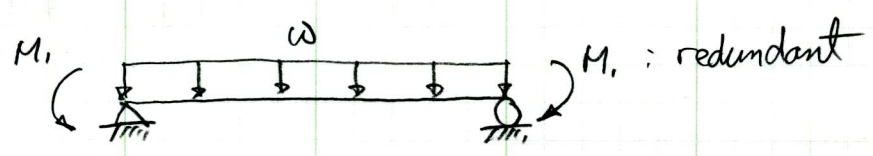
Equilibrium } $M \leq M_p$ } \implies Mechanism Check

\downarrow
Lower bound solution

\downarrow O.K.
Exact plastic limit load



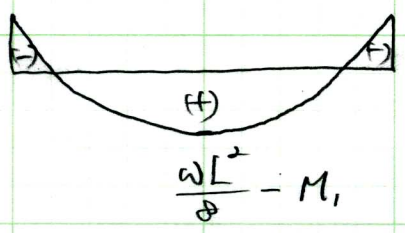
2차 부정경계
 \rightarrow 3개의 plastic hinge 필요



M by applied load



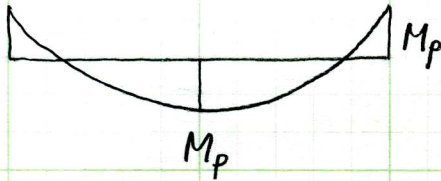
M by redundant



Superimposed M



$$\left\{ \begin{array}{l} M_1 = M_p \\ \frac{\omega L^2}{8} - M_1 = M_p \end{array} \right. \quad \omega^L = \frac{16 M_p}{L^2} \quad \text{Moment assign}$$

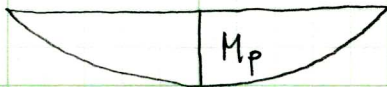


Mechanism Check

\rightarrow O.K. \rightarrow Exact Plastic Limit Load

$$\omega_c = \frac{16 M_p}{L^2}$$

Wrong Assign

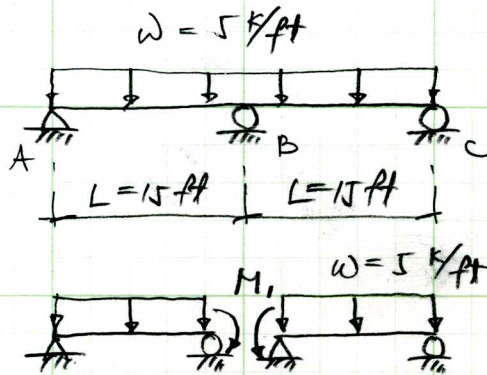


$$M_1 = 0$$

$$\frac{\omega L^2}{8} - M_1 = M_p$$

$$\omega^L = \frac{8 M_p}{L^2} \quad (\text{Too safe})$$

Design Example 1



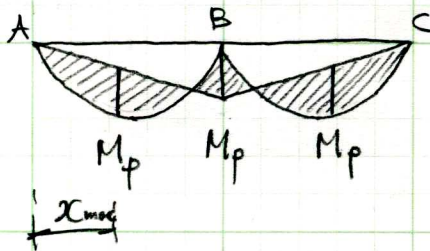
A36

load factor : 1.7

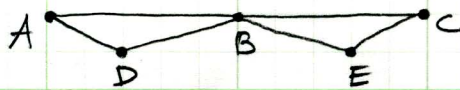
1 차 부정정
⇒ 2 P.H. 필요

W-section ?

M_1 : redundant

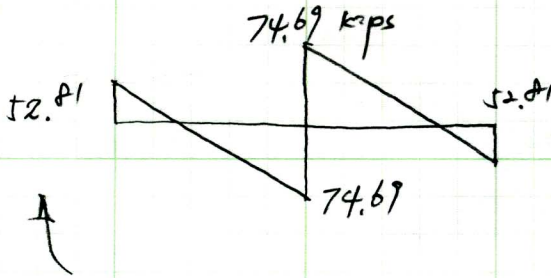


Superimposed M



$$M = \frac{wL}{2}x - \frac{wx^2}{2} - \frac{M_p}{L}x$$

$$\frac{dM}{dx} = 0 \Rightarrow M_{max}$$



$$\frac{dM}{dx} = \frac{wL}{2} - wx - \frac{M_p}{L} = 0$$

$$x = \frac{L}{2} - \frac{M_p}{wL}, \quad M = M_{max} = M_p$$

$$V_A = \frac{(5)(1.7)(15)}{2} - \frac{164.07}{15}$$

$$= 52.81 \text{ kips}$$

$$M_p = \frac{wL^2}{2} [3 - \sqrt{3}]$$

$$= \frac{(5)(1.7)(15)^2}{2} [3 - \sqrt{3}]$$

$$= 164.07 \text{ k-ft}$$

$$Z = \frac{M_p}{F_y} = \frac{(164.07)(12)}{36} = 54.62 \text{ in}^3$$

$$V_B = 74.69 \text{ kips}$$

$$\text{req'd } A_w = \frac{V_B}{\tau_y} = \frac{74.69}{36/\sqrt{3}} = 3.59 \text{ in}^2$$

Try w 16x36

$$Z = 64 \text{ in}^3 > 54.62$$

$$A_w = d_w \cdot t_w = (15.06)(0.29) = 4.42 \text{ in}^2 > 3.59$$

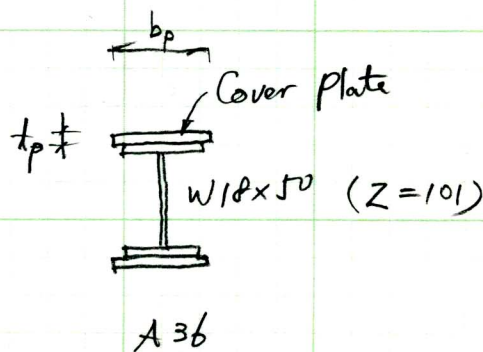
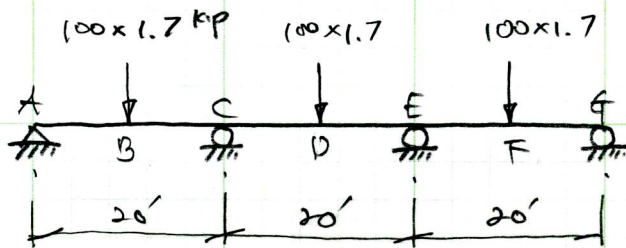
$$\tau = \frac{V_B}{A_w} = \frac{74.69}{4.42} = 16.89 \text{ ksi}$$

$$\begin{aligned} Z_{ps} &= Z - Z_w \left[1 - \frac{\sqrt{\sigma_y - 3\tau^2}}{\sigma_y} \right] \\ &= 64.0 - \frac{(0.29)(15)^2}{4} \left[1 - \frac{\sqrt{(36)^2 - 3(16.89)^2}}{36} \right] \\ &= 64.0 - 6.92 = 57.07 \text{ in}^3 > 54.62 \text{ in}^3 \end{aligned}$$

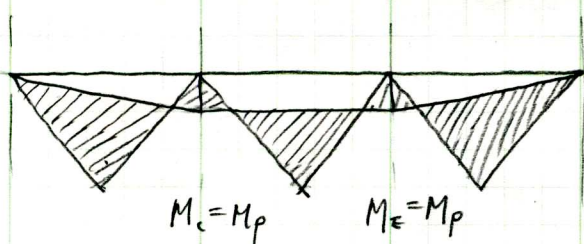
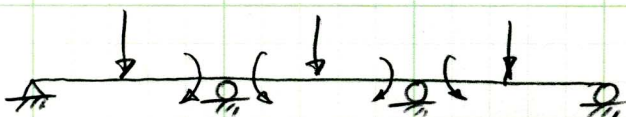
Use w 16x36



Design Example 2



2차부임영



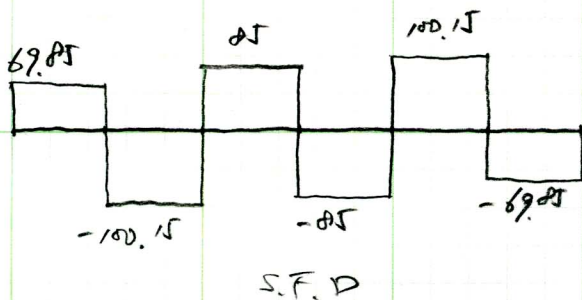
$$\frac{1}{4} \times 100 \times 1.7 \times 20 \times 12 = 10,200$$

$$M_c = M_e = M_p = \sigma_y Z$$

$$= (36) (101) = 3636 \text{ K-in}$$

$$10,200 - \frac{3636}{2} > 3636$$

Cover plate required



$$Z_c = Z_e = \frac{V_c}{A_w} = \frac{100.15}{(16.85)(0.35)} = 16.74 \text{ ksi} < \frac{36}{\sqrt{3}} = 20.8$$

O.K

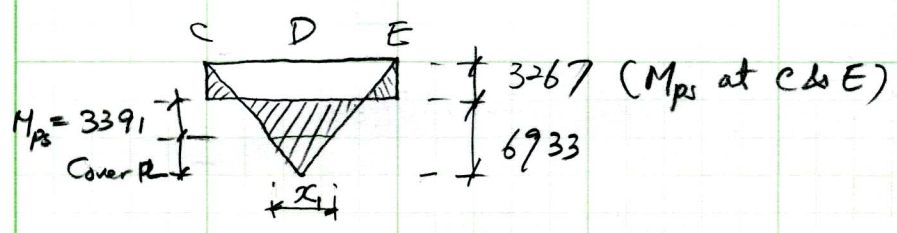
$$M_{ps} = M_p - \sigma_y Z_w \left[1 - \frac{\sqrt{\sigma_y^2 - 3Z^2}}{\sigma_y} \right]$$

$$= 3636 - 36 \left[\frac{0.35 \times 16.85^2}{4} \right] \left[1 - \frac{\sqrt{36^2 - 3(16.74)^2}}{36} \right]$$

$$= 3267 \text{ Kips-in}$$



- Cover Plate for Middle Span



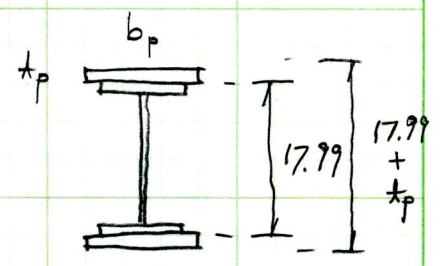
$$M_D = 10,200 - 3267 = 6933$$

$$Z_D = \frac{85}{16.85 \times 0.355} = 14.21$$

$$M_{ps} = 3391 \text{ k-in}$$

$$6933 = 3391 + b_p t_p (17.99 + t_p) (36)$$

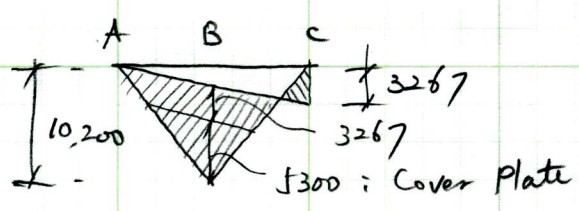
$$\text{Try } t_p = \frac{1}{2}'' \quad b_p = 10.75''$$



$$x_1 = 20 \times \frac{10,200 - 3267 - 3391}{10,200} = 6.95'$$

Use $L = 7'$

- Cover Plate for End Span

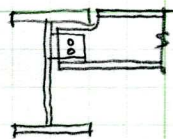
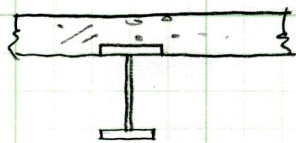


$$Z_{plate} = \frac{300}{36} = t_p b_p (17.99 + t_p)$$

$$\text{try } t_p = \frac{5}{4}'' \rightarrow b_p = 10.54'' \Rightarrow 10\frac{1}{2}'' \quad L = 10.5'$$

- Unbraced Length

Lateral torsional buckling



lateral brace
to develop full plastic
moment.

$$L_p = \frac{300}{\sqrt{F_{yf}}} r_y \quad ; \quad M_p \text{ but no rotation capacity}$$

$$L_{pd} = \frac{3,600 + 2,200 M_1/M_p}{F_y} r_y \quad ; \quad M_p \text{ and enough rotation capacity}$$

Supports and plastic hinge locations ; lateral brace
required

- Check ; Segment AB

$$L_{pd} = \frac{3,600 + 2,200 \times 0/M_p}{36} r_y = 100 r_y = 100 \times \frac{1.65}{12} = 13.75' > 10' \quad \text{O.K.}$$

Check ; Segment BC

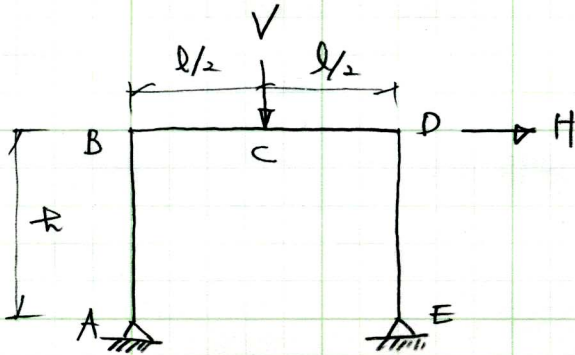
$$L_{pd} = \frac{3600 + 2200 \times \frac{3267}{8567}}{36} \times \frac{1.65}{12} = 16.95' > 10' \quad \text{O.K.}$$

Check ; Segment CD

$$L_{pd} = \frac{3600 + 2200 \times \frac{3267}{6933}}{36} r_y = 17.7' > 10' \quad \text{O.K.}$$



Portal Frame

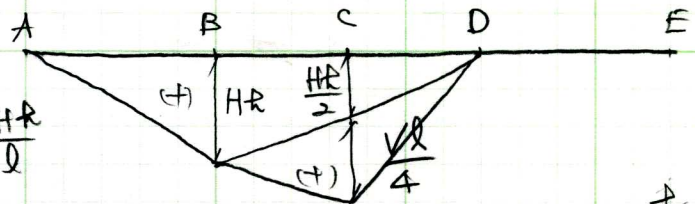
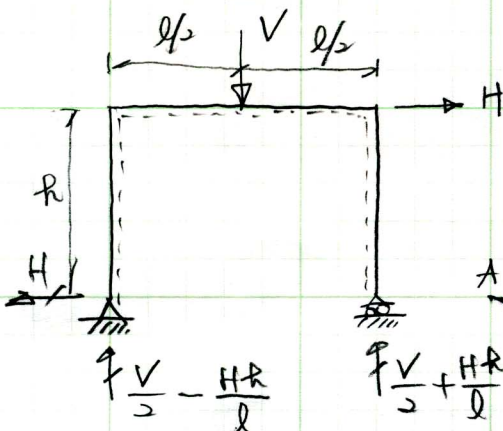


Determine limit value $H = f(M_p)$

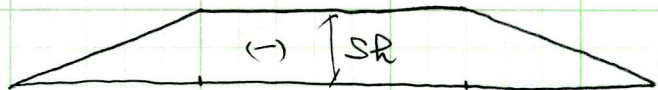
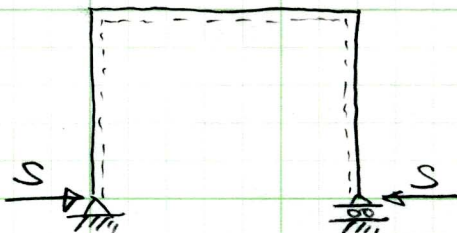
$l/R = 1, V/H = 3 \Rightarrow V = 3H$

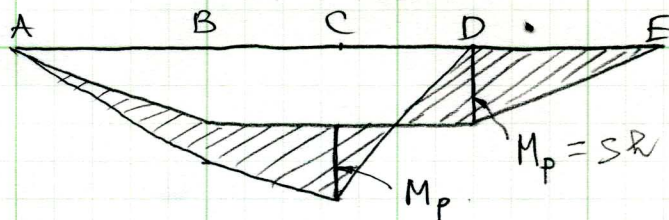
1차 부등강

\Rightarrow 2개의 plastic hinge 필요



$$\frac{HR}{2} + \frac{Vl}{4} = \frac{HR}{2} + \frac{l}{4} 3H = \frac{5}{4} RH$$





$$M_c = \frac{5}{4} Hr - M_p = M_p$$

$$H = 1.6 M_p / r$$

- If $l = r = 20$ ft, W16 x 44

Determine limit value V & H

(a) Without effect of axial force

$$H = 1.6 M_p / r$$

$$M_{px} = Z_x F_y = (12.3)(36) = 2,963 \text{ k-in}$$

$$H = 1.6 \frac{2,963}{(20)(12)} = 19.75 \text{ kips}$$

$$V = 3H = 59.25 \text{ kips}$$

(b) With effect of axial force

• Member BD

$$T_{BD} = H - S = 1.6 \frac{M_p}{r} - \frac{M_p}{r} = 0.6 \frac{M_p}{r} = 7.41 \text{ kips}$$

$$P_y = A F_y = (13.3)(36) = 478.8 \text{ kips}$$

$$\frac{P}{\phi_t P_n} = \frac{7.41}{(0.9)(478.8)} = 0.017 < 0.2$$

$$\frac{P}{\phi_t P_n} + \frac{M_{pc}}{\phi_b M_p} \leq 1.0$$



$$M_{pc} = \left(1 - \frac{0.018}{2}\right) \phi_b M_p = 0.991 \phi_b M_p$$

axial force effect
is negligible

• Member DE

$$P = \frac{V}{2} + \frac{HR}{2} = \frac{V}{2} + H = 49.38 \text{ kips}$$

$$\lambda_{cy} = \frac{1}{\pi} \frac{(KL)_y}{r_y} \sqrt{\frac{F_y}{E}} = \frac{1}{\pi} \frac{(10)(12)}{1.57} \sqrt{\frac{36}{30,000}} = 0.443$$

$$(KL)_y = 10'$$

$$\lambda_{cx} = \frac{1}{\pi} \frac{(KL)_x}{r_x} \sqrt{\frac{F_x}{E}} = \frac{1}{\pi} \frac{(1)(20)(12)}{6.65} \sqrt{\frac{36}{30,000}} = 0.398$$

Weak axis control) $\lambda_c = \lambda_{cy} = 0.443 < 1.5$

$$P_n = 0.658^{\lambda_c^2} P_y = 0.658^{(0.443)^2} (478.8) = 355.6 \text{ kips}$$

$$P/\phi_c P_n = \frac{49.38}{(0.85)(355.6)} = 0.163$$

$$\frac{P}{\phi_c P_n} + \frac{M_{pc}}{\phi_b M_{max}} \leq 1$$

$$\frac{0.163}{2} + \frac{M_{pc}}{0.9 M_p} = 1.0$$

$$M_{pc} = 0.827 M_p$$

$$H = 1.6 \frac{M_{pc}}{R} = \frac{(1.6)(0.827)(2983)}{(20)(12)} = 16.34 \text{ kips}$$

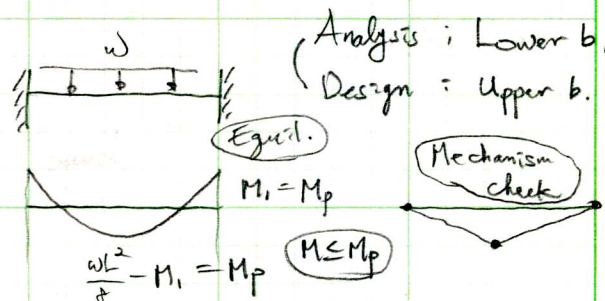
$$V = 3H = 49 \text{ kips}$$

H.W # 4.15

Equilibrium Method

Equilibrium } $M \leq M_p$ } \Rightarrow Mechanism check

Small Structure
 Large Structure



Equilibrium } \Rightarrow Moment Check
 Assume Mechanism

Analysis: Upper bound
 Design: Lower bound

- 1) Select redundants
- 2) M by applied load
- 3) M by redundant forces
- 4) Assume failure mechanism
- 5) M at plastic hinges $= M_p$
- 6) Determine M_p , redundants
- 7) Check $M < M_p$
- 8) Determine upper and lower limits

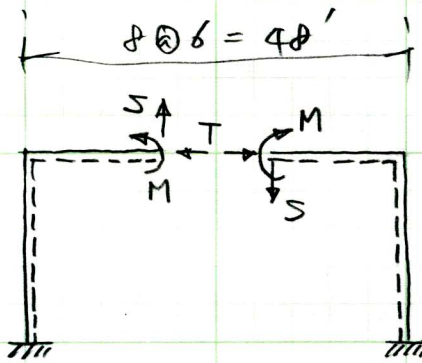
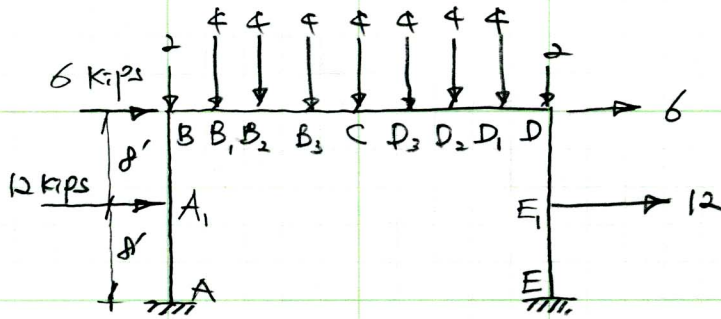


Design Example

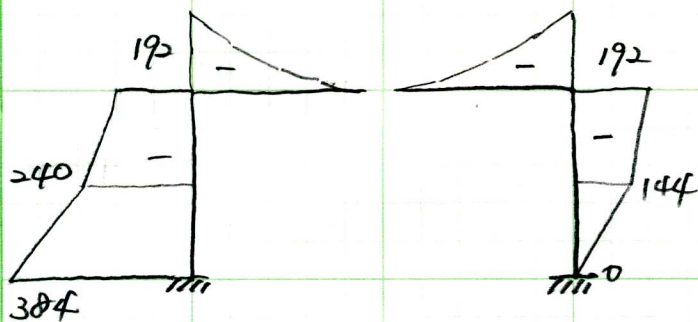
Neglect P, V effect

M_p ?

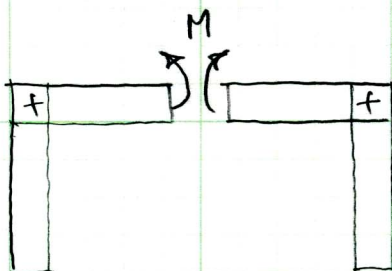
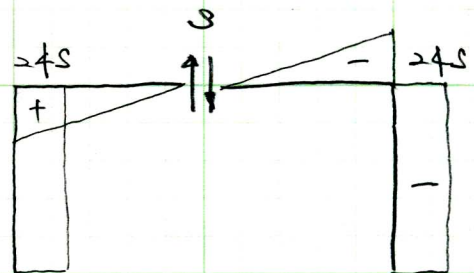
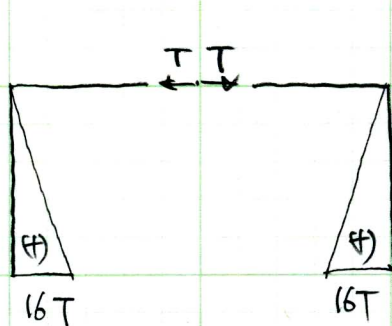
Design Problem



1) Select redundants

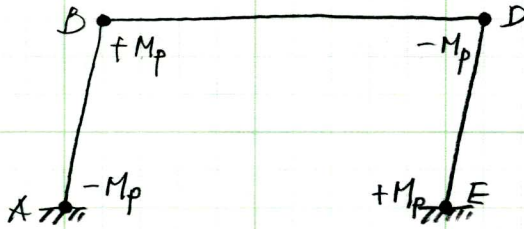


2) M by applied load



3) M by redundant forces

4) Assume failure mechanism 1.



5) M at plastic hinges $= M_p$

$$M_A = -304 + 16T + 24S + M = -M_p$$

$$M_B = -192 + 24S + M = +M_p$$

$$M_D = -192 - 24S + M = -M_p$$

$$M_E = 16T - 24S + M = +M_p$$

Unknown:

M_p, T, S, M ; 4 eq

Equations: 4 eq

6) Determine M_p , redundants

$$M_p = 96 \text{ k-ft}$$

$$M = 192 \text{ k-ft}$$

$$S = 4 \text{ kps}$$

$$T = 0$$

7) Check $M < M_p$

$$M_C = 192 > M_p \quad \text{N.G.} \quad \text{Table 4.1 (p192)}$$

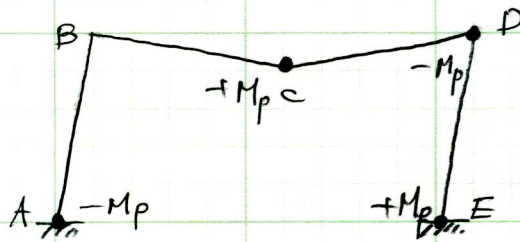
$$M_{B3} = 204 > M_p \quad \text{N.G.}$$

8) Determine upper and lower limits

$$96 \leq M_p \leq 204$$



- For failure mechanism 2.



$$\left\{ \begin{array}{l} M_A = -384 + 16T + 24S + M = -M_p \\ M_C = M = M_p \\ M_D = -192 - 24S + M = -M_p \\ M_E = 16T - 24S + M = M_p \end{array} \right.$$

$$M_p = M = 128 \text{ k-ft}$$

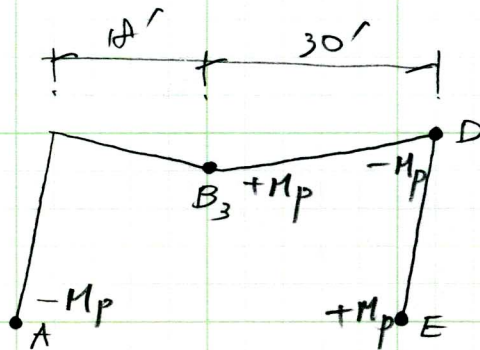
$$S = 2.67 \text{ kips}$$

$$T = 4 \text{ kips}$$

$$M_{B_0} = 132 \text{ kips-ft}$$

$$128 \leq M_p \leq 132$$

- For failure mechanism 3.



$$M_A = -384 + 18T + 24S + M = -M_p$$

$$M_{B_3} = -12 + 6S + M = M_p$$

$$M_D = -192 - 24S + M = -M_p$$

$$M_E = 18T - 24S + M = M_p$$

$$M_p = 129.2 \text{ kip-ft}$$

$$M = 125.6 \text{ kip-ft} < M_p$$

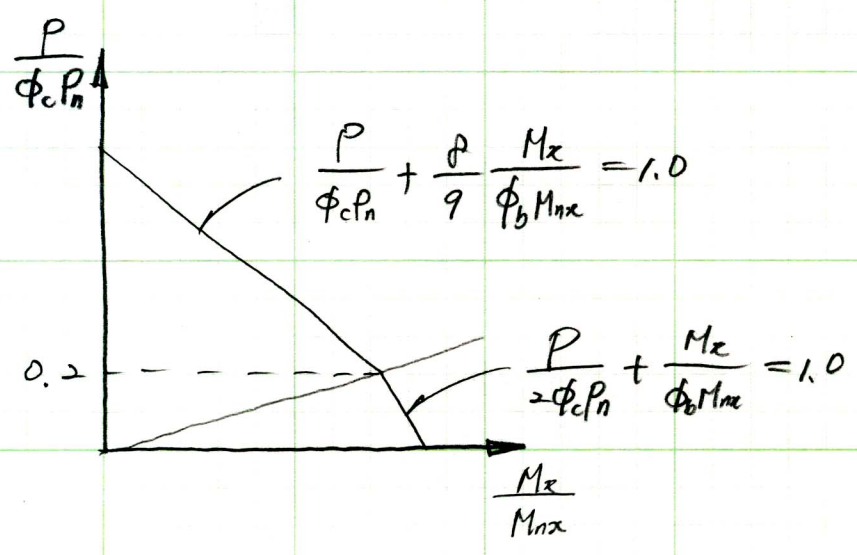
$$S = 2.615 \text{ kips}$$

$$T = 4.15 \text{ kips}$$

$$\therefore M_p = 129.2 \text{ kips}$$



LRFD Check



$$M_x = B_1 M_{nt} + B_2 M_{ot}$$

$$B_1 = \frac{C_m}{1 + \frac{P}{P_{ek}}} \geq 1.0$$

$$C_m = 0.6 - 0.4 \frac{M_A}{M_B}$$

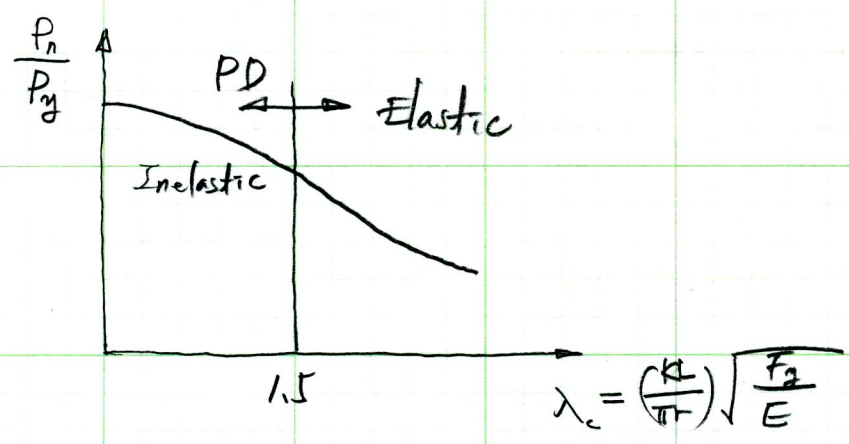
$$C_m = 1 + \psi \frac{P}{P_{ek}}$$

$$B_2 = \frac{1}{1 - \frac{\sum P}{\sum P_{ek}}} \geq 1.0$$

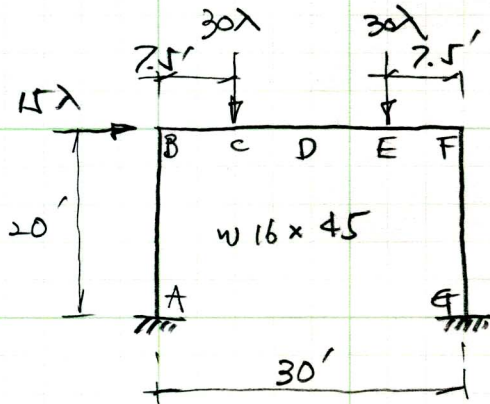
$$P_{ek} = \frac{\pi^2 E}{(K L / r)^2}$$

$$B_2 = \frac{1}{1 - \frac{\sum P \Delta_o}{\sum H L}}$$

K = alignment chart



Analysis Example

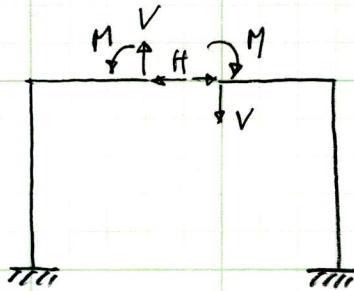


$K_x = K_y = 1$
 $B_1 = B_2 = 1.0$

$\lambda ?$

Analysis problem

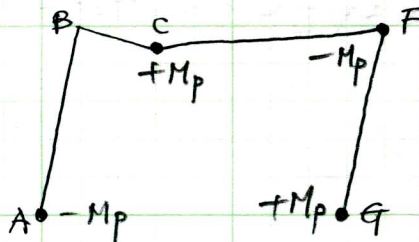
→ Upper bound solution



- M by applied load and redundants

Point	A	B	C	D	E	F	G
Applied Load	-15λ	-25λ	0	0	0	-25λ	-25λ
M	M	M	M	M	M	M	M
V	15V	15V	7.5V	0	-7.5V	-15V	-15V
H	20H	0	0	0	0	0	20H

- Assume Mechanism



- Equilibrium

$$M_A = -15\lambda + M + 15V + 20H = -M_p$$

$$M_C = M + 7.5V = M_p$$

$$M_F = -25\lambda + M - 15V = -M_p$$

$$M_G = -25\lambda + M - 15V + 20H = M_p$$

$$H = \frac{M_p}{10}$$

$$\lambda = \frac{7}{900} M_p$$

$$V = \frac{M_p}{90}$$

$$M = \frac{11}{12} M_p$$

If M at a point is greater than M_p , the lower bound solution is $\lambda = \lambda \frac{M_p}{M}$

- Moment check

$$M_B = -225\lambda + M + 15V = -\frac{2}{3}M_p < M_p \quad \text{O.K.}$$

$$M_D = M = \frac{11}{12} M_p < M_p \quad \text{O.K.}$$

$$M_E = M - 7.5V = \frac{10}{12} M_p < M_p \quad \text{O.K.}$$

- Effect of axial load is neglected

$$\lambda = \frac{7}{900} M_p = \frac{7}{900} (247) = 1.92$$

- Axial force is considered

$$P_{Fy} = 30\lambda + V = 30\lambda + \frac{M_p}{90} = 30(1.92) + \frac{247}{90} = 60.3 \text{ kips}$$

$$P_y = (13.3)(36) = 478.8 \text{ kips}$$

$$\lambda_{cx} = \frac{1}{\pi} \frac{KL}{r_x} \sqrt{\frac{F_c}{E}} = \frac{1}{\pi} \frac{(80)(11)}{6.65} \sqrt{\frac{36}{30,000}} = 0.398$$

$$\lambda_{cy} = \frac{1}{\pi} \frac{(20)(12)}{1.57} \sqrt{\frac{36}{30,000}} = 1.686$$

$$P_n = \left(\frac{0.877}{\lambda_c^2} \right) P_y = 147.7 \text{ kips}$$

for plastic design $\lambda_c < 1.5$



$$\lambda_c = \frac{1.686}{4} = 0.422 < 1.5, \text{ O.K.}$$

$$F_n = 0.658 \lambda_c^2 F_y = 444.4 \text{ kips}$$

$$\frac{P}{\phi_c P_n} = \frac{60.3}{(0.85)(444.4)} = 0.160 < 0.2$$

$$\frac{P}{2\phi_c P_n} + \frac{M_x}{\phi_b M_{nx}} \leq 1.0$$

$$\phi_b = 0.9, \quad M_{nx} = M_{px} = 2963 \text{ k-in}, \quad M_x = M_{pc}$$

$$\frac{M_x}{\phi_b M_{nx}} = \frac{M_{pc}}{0.9 M_p} \leq \left(1 - \frac{0.160}{2}\right)$$

$$M_{pc} = (0.92)(0.9) M_p = 0.828 M_p$$

$$\therefore \lambda = (0.92)(0.828) = 1.59$$

- Shear Force considered

$$V_p = 0.55 F_y t_w d = 0.55 \times 36 \times 0.34 \times 16.13 = 110 \text{ kips}$$

$$V_{ef} = 30\lambda + V = 60.3 \text{ kips} < V_p$$

⇒ M_p may be used.

H.W # 7 4.19