

WRITE DOWN

## Chapter 7

### First-Order Hinge by Hinge Analysis

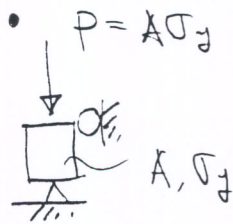
- Basic Idea
  - Problem Definition : Example problem
  - Program Installation
  - Structure of Program
  - Creating Input File
  - Program Execution
  - Output Interpretation
  - Homework
  - Basic Formulation
- How to use program  
Very straight forward

# o First - Order / Second - Order

## First - Order

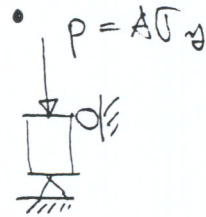
(Chapter 7, FOIPA)

### Stocky Member

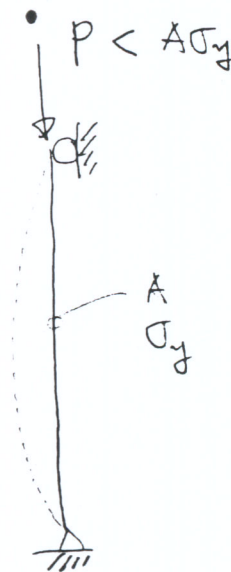
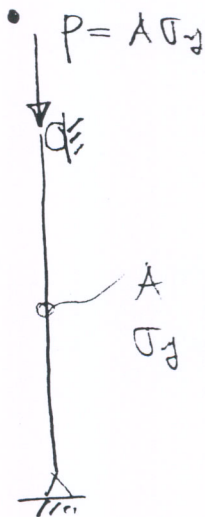


## Second - Order

(Chapter 8, PHINGE)



### Slender member



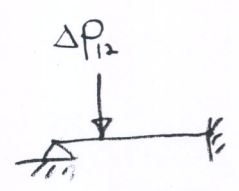
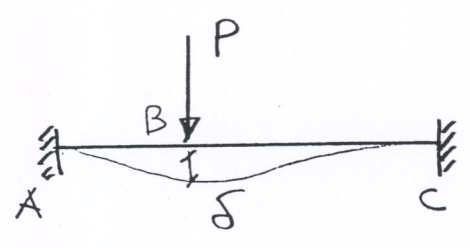
does not consider  
buckling effect  
which depends on  
slenderness

- consider  
buckling effect  
which depends on  
slenderness

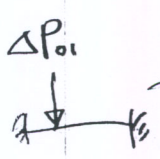
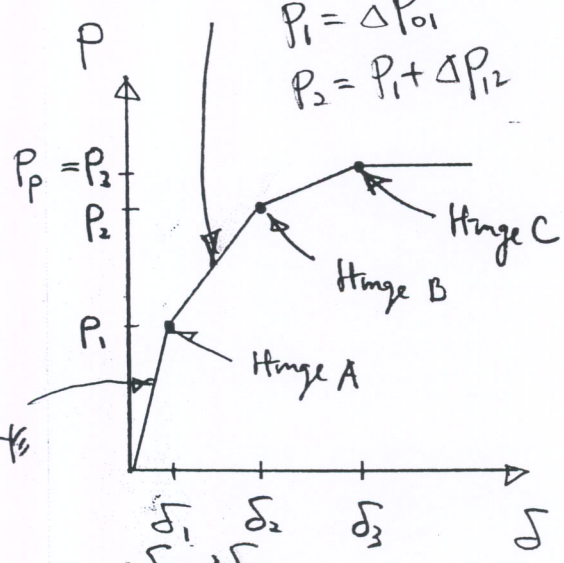
\* More realistic

o Basic Idea

For simple structure



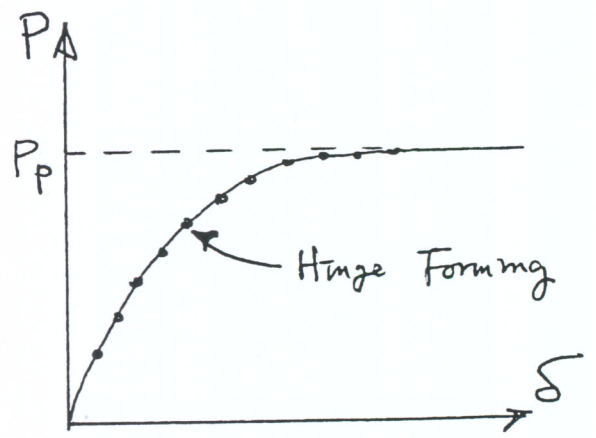
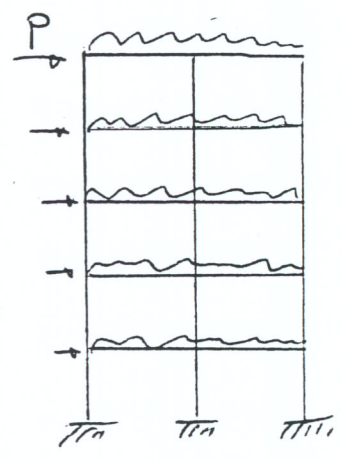
$P_1 = \Delta P_{01}$   
 $P_2 = P_1 + \Delta P_{12}$



$\delta_1 = \Delta \delta_{01}$   
 $\delta_2 = \delta_1 + \Delta \delta_{12}$

- \* Hand calculation is possible
- \* Chapter 1.

For large structure



- \* Computer program is necessary
- \* Chapter 7. (FOPA)

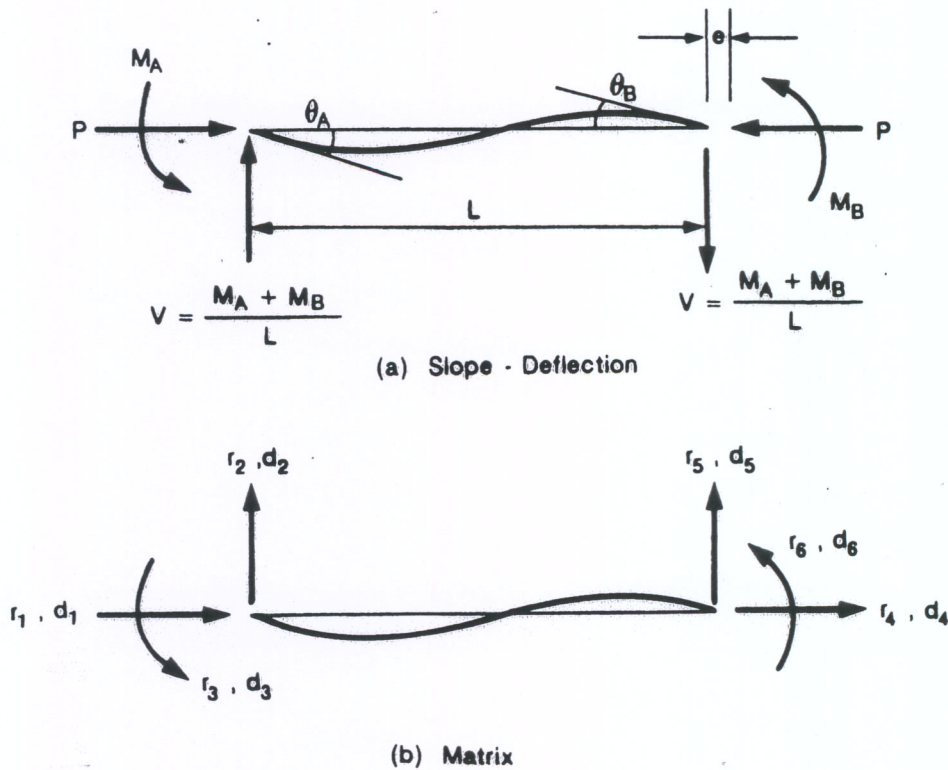


FIGURE 7.2. (a) Slope-deflection and (b) matrix analysis coordinates.

$$\begin{Bmatrix} M_A \\ M_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{Bmatrix} \theta_A \\ \theta_B \end{Bmatrix}. \quad (7.2.10)$$

These are the well-known *slope-deflection* equations. However, in a usual structural analysis, it is more convenient to use a matrix analysis coordinate. This transformation can be achieved by simply comparing member end forces and end displacement in the slope-deflection coordinate with the matrix analysis coordinate as shown in Fig. 7.2. The equilibrium relationship between the end forces in these two systems has the form

$$\begin{Bmatrix} r_1 \\ r_2 \\ r_3 \\ r_4 \\ r_5 \\ r_6 \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & \frac{1}{L} & \frac{1}{L} \\ 0 & 1 & 1 \\ -1 & 0 & 0 \\ 0 & -\frac{1}{L} & -\frac{1}{L} \\ 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} P \\ M_A \\ M_B \end{Bmatrix}. \quad (7.2.11)$$



The kinematic relationship between the end displacements in these two systems is

$$\begin{Bmatrix} e \\ \theta_A \\ \theta_B \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 & -1 & 0 & 0 \\ 0 & \frac{1}{L} & 1 & 0 & -\frac{1}{L} & 0 \\ 0 & \frac{1}{L} & 0 & 0 & -\frac{1}{L} & 1 \end{bmatrix} \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{Bmatrix}. \quad (7.2.12)$$

By including the axial force and axial deformation relationship in the slope-deflection equation (7.2.10), the new slope-deflection equation can be expressed as

$$\begin{Bmatrix} P \\ M_A \\ M_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} \frac{A}{I} & 0 & 0 \\ 0 & 4 & 2 \\ 0 & 2 & 4 \end{bmatrix} \begin{Bmatrix} e \\ \theta_A \\ \theta_B \end{Bmatrix}. \quad (7.2.13)$$

The relationship between the end forces  $\{r\}$  and the end displacements  $\{d\}$  can now be obtained by substituting Eq. (7.2.13) into Eq. (7.2.11) and substituting Eq. (7.2.12) in the resulting equation as

$$\begin{Bmatrix} r_1 \\ r_2 \\ r_3 \\ r_4 \\ r_5 \\ r_6 \end{Bmatrix} = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{12EI}{L^3} & \frac{6EI}{L^2} & 0 & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{4EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & 0 & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{2EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{Bmatrix}. \quad (7.2.14)$$

### 7.3 Stiffness Matrix for a Beam Element with a Plastic Hinge at End A

When one plastic hinge is formed at end A, then the incremental moment at end A is zero. Using the condition that  $M_A = 0$  in the first matrix Eq. (7.2.10), we obtain the kinematic relation in the incremental form as  $\dot{\theta}_A = -\dot{\theta}_B/2$ .

Substituting this  $\dot{\theta}_A$  into the second equation of matrix Eq. (7.2.10) and then rewriting it in matrix form, we have

$$\begin{Bmatrix} \dot{M}_A \\ \dot{M}_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} 0 & 0 \\ 0 & 3 \end{bmatrix} \begin{Bmatrix} \dot{\theta}_A \\ \dot{\theta}_B \end{Bmatrix}. \quad (7.3.1)$$

Including the incremental axial load and axial-deformation relationship in Eq. (7.3.1), we have

$$\begin{Bmatrix} \dot{P} \\ \dot{M}_A \\ \dot{M}_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} \frac{A}{I} & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 3 \end{bmatrix} \begin{Bmatrix} \dot{e} \\ \dot{\theta}_A \\ \dot{\theta}_B \end{Bmatrix}. \quad (7.3.2)$$

This relationship can now be transformed into the matrix analysis coordinate by using the end forces relation (7.2.11) and the kinematic relation (7.2.12) in incremental form as

$$\begin{Bmatrix} \dot{r}_1 \\ \dot{r}_2 \\ \dot{r}_3 \\ \dot{r}_4 \\ \dot{r}_5 \\ \dot{r}_6 \end{Bmatrix} = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{3EI}{L^3} & 0 & 0 & -\frac{3EI}{L^3} & \frac{3EI}{L^2} \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{3EI}{L^3} & 0 & 0 & \frac{3EI}{L^3} & -\frac{3EI}{L^2} \\ 0 & \frac{3EI}{L^2} & 0 & 0 & -\frac{3EI}{L^2} & \frac{3EI}{L} \end{bmatrix} \begin{Bmatrix} \dot{d}_1 \\ \dot{d}_2 \\ \dot{d}_3 \\ \dot{d}_4 \\ \dot{d}_5 \\ \dot{d}_6 \end{Bmatrix}. \quad (7.3.3)$$

#### 7.4 Stiffness Matrix for a Beam Element with a Plastic Hinge at End B

The incremental slope-deflection relation for this case can be derived in a similar way to that of the previous case as

$$\begin{Bmatrix} \dot{M}_A \\ \dot{M}_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} 3 & 0 \\ 0 & 0 \end{bmatrix} \begin{Bmatrix} \dot{\theta}_A \\ \dot{\theta}_B \end{Bmatrix}. \quad (7.4.1)$$

Including the incremental axial load-axial deformation relation in Eq. (7.4.1), we have

$$\begin{Bmatrix} \dot{P} \\ \dot{M}_A \\ \dot{M}_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} \frac{A}{I} & 0 & 0 \\ 0 & 3 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} \dot{e} \\ \dot{\theta}_A \\ \dot{\theta}_B \end{Bmatrix}. \quad (7.4.2)$$



Using Eqs. (7.2.11) and (7.2.12) in an incremental form for the transformation to a matrix analysis coordinate, we have

$$\begin{Bmatrix} \dot{r}_1 \\ \dot{r}_2 \\ \dot{r}_3 \\ \dot{r}_4 \\ \dot{r}_5 \\ \dot{r}_6 \end{Bmatrix} = \begin{bmatrix} \frac{AE}{L} & 0 & 0 & -\frac{AE}{L} & 0 & 0 \\ 0 & \frac{3EI}{L^3} & \frac{3EI}{L^2} & 0 & -\frac{3EI}{L^3} & 0 \\ 0 & \frac{3EI}{L^2} & \frac{3EI}{L^2} & 0 & -\frac{3EI}{L^2} & 0 \\ -\frac{AE}{L} & 0 & 0 & \frac{AE}{L} & 0 & 0 \\ 0 & -\frac{3EI}{L^3} & -\frac{3EI}{L^2} & 0 & \frac{3EI}{L^3} & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} \dot{d}_1 \\ \dot{d}_2 \\ \dot{d}_3 \\ \dot{d}_4 \\ \dot{d}_5 \\ \dot{d}_6 \end{Bmatrix} \quad (7.4.3)$$

### 7.5 Plastic Hinges at Both Ends A and B

When plastic hinges are formed at both ends, no additional moment can be applied at these ends, and the incremental slope-deflection relation becomes

$$\begin{Bmatrix} \dot{M}_A \\ \dot{M}_B \end{Bmatrix} = \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix} \begin{Bmatrix} \dot{\theta}_A \\ \dot{\theta}_B \end{Bmatrix} \quad (7.5.1)$$

After transformation to a matrix analysis coordinate, the incremental force-displacement relation becomes

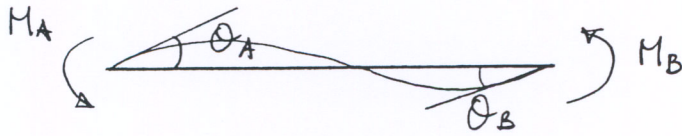
$$\begin{Bmatrix} \dot{r}_1 \\ \dot{r}_2 \\ \dot{r}_3 \\ \dot{r}_4 \\ \dot{r}_5 \\ \dot{r}_6 \end{Bmatrix} = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} \dot{d}_1 \\ \dot{d}_2 \\ \dot{d}_3 \\ \dot{d}_4 \\ \dot{d}_5 \\ \dot{d}_6 \end{Bmatrix} \quad (7.5.2)$$

### 7.6 Stiffness Matrix for a Beam with an Intermediate Plastic Hinge

To derive the stiffness matrix for a beam with an intermediate plastic hinge (Fig. 7.3), we will use a slightly different approach. The stiffness matrix is obtained directly in the matrix analysis coordinates by applying a unit displacement along the specified degree of freedom. The corresponding stiffness

o Basic Formulation

- Slope - Deflection Equation

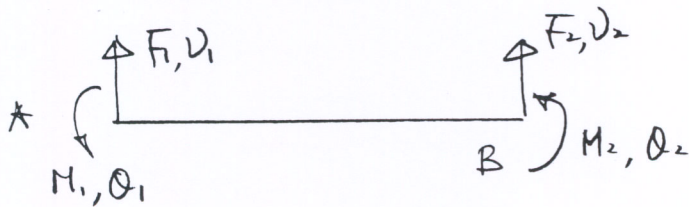


$$M_A = \frac{4EI}{L} \theta_A + \frac{2EI}{L} \theta_B$$

$$M_B = \frac{2EI}{L} \theta_A + \frac{4EI}{L} \theta_B$$

$$\begin{Bmatrix} M_A \\ M_B \end{Bmatrix} = \frac{EI}{L} \begin{bmatrix} 4 & 2 \\ 2 & 4 \end{bmatrix} \begin{Bmatrix} \theta_A \\ \theta_B \end{Bmatrix}$$

- Beam Element

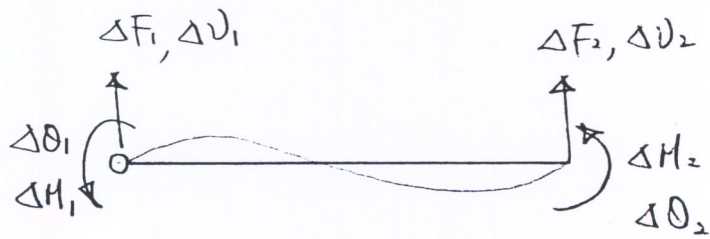


$$\begin{Bmatrix} F_1 \\ M_1 \\ F_2 \\ M_2 \end{Bmatrix} = \begin{bmatrix} \frac{12EI}{L^3} & \frac{6EI}{L^2} & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ & \frac{4EI}{L} & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ \text{Sym.} & & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ & & & \frac{4EI}{L} \end{bmatrix} \begin{Bmatrix} \theta_1 \\ \theta_2 \\ \theta_1 \\ \theta_2 \end{Bmatrix}$$

$$F = K \times \delta$$



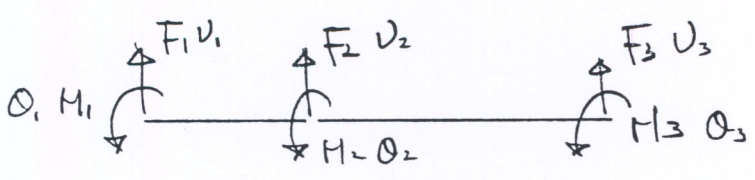
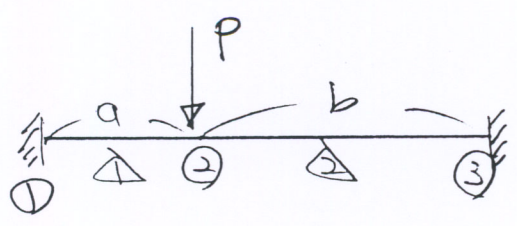
- If Plastic hinge at A



$$\begin{bmatrix} \Delta F_1 \\ \Delta H_1 \\ \Delta F_2 \\ \Delta H_2 \end{bmatrix} = \begin{bmatrix} \frac{3EI}{L^3} & 0 & -\frac{3EI}{L^3} & \frac{3EI}{L^2} \\ 0 & 0 & 0 & 0 \\ \frac{3EI}{L^3} & 0 & \frac{3EI}{L^3} & -\frac{3EI}{L^2} \\ \text{Sym.} & & \frac{3EI}{L} & \frac{3EI}{L} \end{bmatrix} \begin{bmatrix} \Delta v_1 \\ \Delta \theta_1 \\ \Delta v_2 \\ \Delta \theta_2 \end{bmatrix}$$

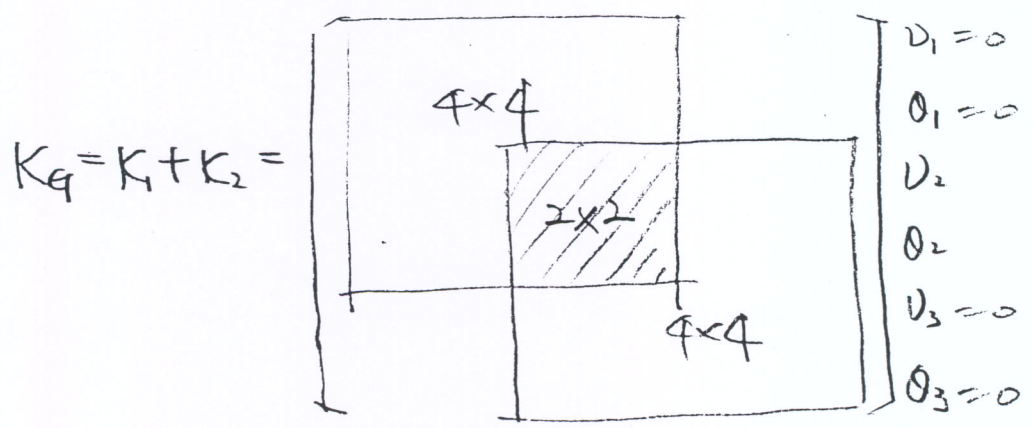
$$\Delta F = K \times \Delta S$$

- Solution Procedure For Original Structure

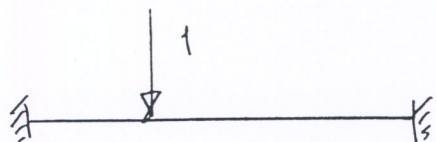


$$K_1 = \begin{bmatrix} \frac{12EI}{a^3} & \frac{6EI}{a^2} \\ \dots & \dots \\ \dots & \dots \\ \frac{6EI}{a} & \dots \end{bmatrix} \quad (4 \times 4)$$

$$K_2 = \begin{bmatrix} \frac{12EI}{b^3} & \frac{6EI}{b^2} \\ \dots & \dots \\ \dots & \dots \\ \frac{6EI}{b} & \dots \end{bmatrix} \quad (4 \times 4)$$



$$\begin{Bmatrix} F_2 \\ M_2 \end{Bmatrix} = \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{Bmatrix} v_2 \\ \theta_2 \end{Bmatrix}$$



$$\begin{Bmatrix} -1 \\ 0 \end{Bmatrix} = \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{Bmatrix} v_2 \\ \theta_2 \end{Bmatrix} \quad \underline{\text{Simultaneous } E_2}$$

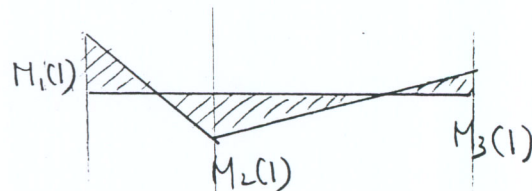
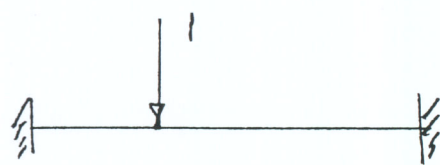
↑  
Unknown.

$$\begin{Bmatrix} v_2 \\ \theta_2 \end{Bmatrix} = \begin{Bmatrix} \dots \\ \dots \end{Bmatrix}$$

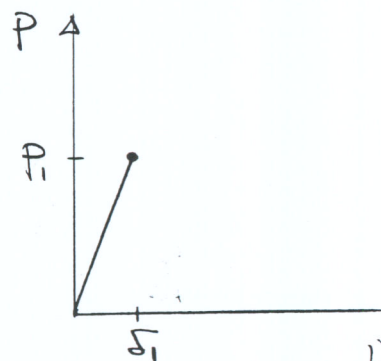
Back-substitution

$$\begin{Bmatrix} F_1 \\ M_1 \\ F_2 \\ M_2 \end{Bmatrix} = \begin{bmatrix} K_1 \end{bmatrix} \begin{Bmatrix} 0 \\ 0 \\ v_2 \\ \theta_2 \end{Bmatrix} = \begin{Bmatrix} \dots \\ \dots \\ \dots \\ \dots \end{Bmatrix}$$

$$\begin{Bmatrix} F_2 \\ M_2 \\ F_3 \\ M_3 \end{Bmatrix} = \begin{bmatrix} K_2 \end{bmatrix} \begin{Bmatrix} v_2 \\ \theta_2 \\ 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} \dots \\ \dots \\ \dots \\ \dots \end{Bmatrix}$$



Max.  $(M_1, M_2, M_3) \Rightarrow M_1 \quad \lambda_1 = \frac{M_p}{M_1(l)}$



Amplification

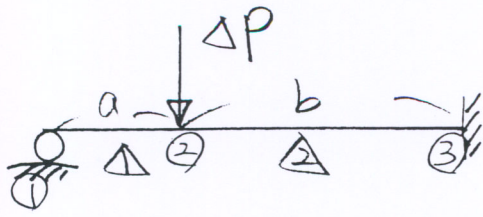
$$P_1(l) = 1 \quad \Rightarrow \quad \lambda_1 P_1(l) = P_1$$

$$M_1(l), M_2(l), M_3(l) \quad \Rightarrow \quad \lambda_1 M_1(l) = M_1$$

$$v_2(l) \quad \Rightarrow \quad \lambda_1 v_2(l) = v_2$$



- Solution Procedure After Hinge Forming

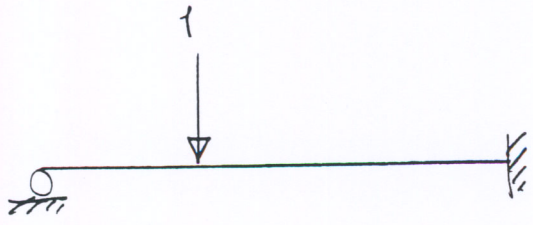


$$K_1 = \begin{bmatrix} \frac{3EI}{a^3} & & \frac{3EI}{a^2} \\ & 0 & 0 \\ & & \frac{3EI}{a} \end{bmatrix}$$

$$K_2 = \begin{bmatrix} \frac{12EI}{b^3} & & \frac{6EI}{b^2} \\ & & \vdots \\ & & \frac{4EI}{b} \end{bmatrix}$$

$$K_G = K_1 + K_2 = \begin{bmatrix} 4 \times 4 & & & & & \\ & 2 \times 2 & & & & \\ & & 4 \times 4 & & & \\ & & & & & \\ & & & & & \end{bmatrix} \begin{matrix} U_1 = 0 \\ \theta_1 = \dots \\ U_2 \\ \theta_2 \\ U_3 = 0 \\ \theta_3 = 0 \end{matrix}$$

$$\begin{Bmatrix} \Delta F_2 \\ \Delta H_2 \end{Bmatrix} = \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{Bmatrix} \Delta U_2 \\ \Delta \theta_2 \end{Bmatrix}$$

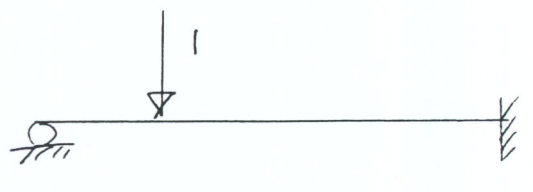


$$\begin{Bmatrix} -1 \\ 0 \end{Bmatrix} = \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{Bmatrix} \Delta v_2 \\ \Delta \theta_2 \end{Bmatrix}$$

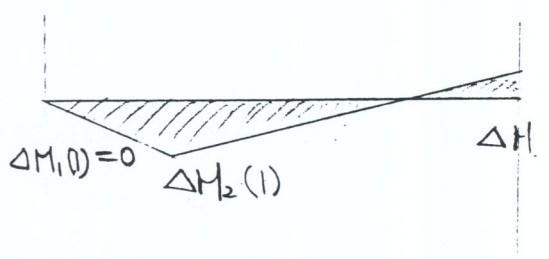
$$\begin{Bmatrix} \Delta v_2 \\ \Delta \theta_2 \end{Bmatrix} = \begin{Bmatrix} \dots \\ \dots \end{Bmatrix}$$

Back-Substitution

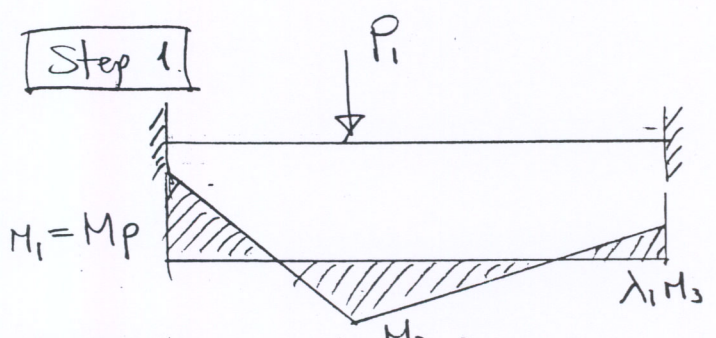
$$\begin{Bmatrix} \Delta F_1 \\ \Delta M_1 \\ \Delta F_2 \\ \Delta M_2 \end{Bmatrix} = \begin{bmatrix} K_1 \end{bmatrix} \begin{Bmatrix} 0 \\ \dots \\ \Delta v_2 \\ \Delta \theta_2 \end{Bmatrix} = \begin{Bmatrix} \dots \\ 0 \\ \dots \\ \dots \end{Bmatrix}$$



$$\begin{Bmatrix} \Delta F_2 \\ \Delta M_2 \\ \Delta F_3 \\ \Delta M_3 \end{Bmatrix} = \begin{bmatrix} K_2 \end{bmatrix} \begin{Bmatrix} \Delta v_2 \\ \Delta \theta_2 \\ 0 \\ 0 \end{Bmatrix} = \begin{Bmatrix} \dots \\ \dots \\ \dots \\ \dots \end{Bmatrix}$$



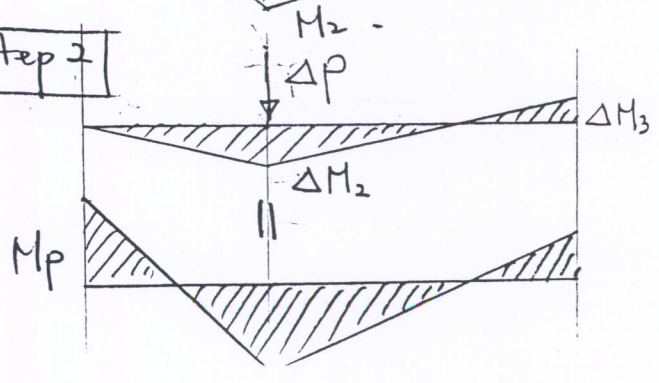
Step 1



$$M_2 + [\Delta \lambda] [\Delta M_2(1)] = M_p$$

$$\Delta \lambda = \frac{M_p - M_2}{\Delta M_2(1)}$$

Step 2



$$\lambda_2 = \lambda_1 + \Delta \lambda$$

# Amplification

$$\Delta P(\omega) = 1 \Rightarrow \Delta \lambda \times 1 = \Delta P$$

$$P_2 = P_1 + \Delta P$$

$$\Delta M_1(\omega) \Rightarrow \Delta \lambda \times \Delta M_1(\omega) = \Delta M_1$$

$$\Delta V_2(\omega) \Rightarrow \Delta \lambda \times \Delta V_2(\omega) = \Delta V_2$$

